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The use of pure oxygen for aeration in aerobic wastewater treatment: a review of its potential and limitations

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1	THE USE OF PURE OXYGEN FOR AERATION IN AEROBIC WASTEWATER
2	TREATMENT: A REVIEW ON ITS POTENTIAL AND LIMITATIONS
3	
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5	
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11	
12	Abstract
13	
14	In aerobic wastewater treatment, aeration is the most critical element of the treatment
15	system. It supplies microorganisms with the required dissolved oxygen, maintains solids in
16	suspension and, in membrane bioreactors, it controls fouling. However, conventional
17	activated sludge is limited to the treatment of low strength wastewaters, as higher loadings
18	require both higher biomass and higher dissolved oxygen concentrations. By replacing air
19	with pure oxygen, oxygen transfer rates increase at lower flowrates. In this work, the
20	potential and limitations of pure oxygen aeration are reviewed. The effect of the system's
21	operational parameters and the mixed liquor characteristics on oxygen transfer, and vice
22	versa, is determined. Pure oxygen treats higher loadings without compromising effluent
23	quality. Fine bubbles are more efficient in oxygen transfer due to their increased contact

area. However, pure oxygen use is not always essential, so we recommend that it berestricted to applications where air use is not adequate.

26

Key words: aerobic treatment, activated sludge, membrane bioreactor, pure oxygen, finebubbles

29

30 1. INTRODUCTION

31

Secondary wastewater treatment (WWT) is an effective and cheap method for removing 32 organic pollutants from wastewaters. Conventional activated sludge (CAS), an aerobic 33 suspended growth treatment process, is one of the most widely used secondary treatment 34 technologies. CAS requires oxygen for the microbial consortia to assist them in degrading 35 the organic matter in wastewater ensuring their maintenance and growth. This makes 36 37 aeration systems an integral part of the CAS plants, (Zhang et al., 2019; Calderón et al., 2013). Aeration supplies the dissolved oxygen (DO) that is required by the biomass in both 38 CAS and membrane bioreactor (MBR) systems with MBRs being activated sludge systems 39 40 where membrane filtration has replaced gravitational sedimentation. It also maintains solids in suspension and, in membrane bioreactors, it additionally mitigates membrane fouling 41 42 and improves membrane cleaning, (Calderón et al., 2012). However, it is an energy 43 intensive process accounting for the largest fraction (40%-75%) of energy costs of a WWT 44 plant. In addition, existing technologies are old, often operating at a standard oxygen 45 transfer efficiency (SOTE) of <15% or at even lower values, after real DO concentrations 46 are considered, (Syron et al., 2015).

47	In principle, treatment efficiency in CAS is controlled by the DO amount in the aeration
48	tank. Low DO concentrations lead to lower effluent quality due to low growth rates of
49	bacteria. Population of filamentous microorganisms in the sludge also increases, which
50	causes sludge bulking or biological foam (froth) formation, (Xu et al., 2016; Jolis et al.,
51	2006; Lee et al., 2003). When low strength wastewaters are treated, traditional oxygen
52	supply with air does satisfy the need for oxygen. However, CAS fails to treat high strength
53	wastewaters as any increase in either the oxygen transfer efficiency (OTE) or in the
54	biomass concentration within the aeration tank, is limited, (Lee and Kim, 2003).
55	As such, when high strength wastewaters are to be treated, satisfactory oxygen demand
56	requires an intensified oxygen supply technology, e.g. pressurized aeration, deep shaft
57	technology, pure oxygen (PO) use. In doing so, high mixed liquor suspended solids
58	(MLSS) concentrations are combined with high OTEs. Pressurization accelerates the
59	oxygen transfer rates (OTRs), or the actual mass of oxygen transferred per unit time, by
60	increasing the partial pressure of oxygen. This is a low cost and a small footprint technique
61	that ensures improved oxygen transfer. Deep shaft aeration employs hydrostatic pressure in
62	a deep well to achieve better OTEs and is a technique that does not need much energy or
63	space. Mass transfer depends on the depth of the well and the technique is widely used in
64	plants treating high strength wastewaters. Finally, regarding PO aeration, PO (>90%)
65	replaces air in aeration and maintains good aerobic conditions even when high organic
66	loadings are treated, (Zhang et al., 2019; Xu et al., 2016; Stenstrom and Rosso, 2010;
67	Esparza-Soto et al., 2006a; Lee and Kim, 2003). In this work, we concentrate on PO
68	aeration and analyze its effect on WWT.
69	PO was first proposed in 1940 as a replacement of air in CAS but it was not put into

commercial use until the '70s in the USA. The first PO activated sludge (POAS) plants for

71	municipal effluent treatment were introduced in 1968, (Zhang et al., 2019; Calderón et al.,
72	2013, 2012). To date, various kinds of wastewater have been treated by treatment systems
73	incorporating PO aeration, (Zhang et al., 2019), including industrial wastewaters that
74	contain toxic and refractory contaminants, (Zhuang et al., 2016b), and landfill leachates,
75	(Canziani et al., 2006). In addition, PO has already been used in MBRs, (Rodríguez et al.,
76	2010), biofilms (BFs) and moving bed biofilm reactors (MBBRs), (Salvetti et al., 2006), or
77	in aerobic granulation applications, (Pan et al., 2017). As of 2014, in the USA, as much as
78	15% of all municipal WWT was realized with the use of PO in aerobic WWT plants,
79	(Larrea et al., 2014). Table 1 shows research studies based on PO aeration and Table 2
80	shows large scale POAS applications.
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83 84	TABLE 2: Industrial WWT POAS applications
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95	stripping, so they reduce odour and volatile organic compound (VOC) emissions, decrease	;
96	sludge production, as more complete oxidization to CO2 is achieved, and minimize sludge	
97	bulking and biomass foaming problems. PO ensures treatment system stability at reduced	
98	power consumption rates and lower sludge disposal costs, (Neerackal et al., 2016; Zhuang	
99	et al., 2016b; Larrea et al., 2014; Calderón et al., 2013; Rodríguez et al., 2012b; Paice et al	••
100	2003; Brindle et al., 1998; Shelef and Green, 1980). However, both oxygen generation	
101	equipment and the purchase of oxygen tanks have high costs, so oxygen should be utilized	
102	at minimum wastage, (Calderon et al., 2012; Brindle et al., 1998).	
103	We herein give an overview on the potential effect of the replacement of air with PO in	
104	aerobic treatment methods. We summarize the consequences of the applied operational	
105	parameters and the influent and biomass characteristics on oxygen transfer, and vice versa	,
106	i.e. the consequences of PO on the treatment process over time. This work deals with PO	
107	introduced into tanks in bubbly form, which then dissolves into the liquid. This review has	
108	covered much of the existing literature and has dealt with a topic that has not been	
109	thoroughly reviewed to date. In addition, it gives a detailed assessment of the use of PO in	
110	both CAS systems and MBRs, comparing the two technologies, where possible.	
111		
112	2. PURE OXYGEN AERATION IN AEROBIC WASTEWATER TREATMENT	
113		
114	2.1. Analysis of Pure Oxygen Aeration	
115		
116	Oxygen transfer rates depend both on the driving force, namely the difference of the DO	
117	saturation concentration in water minus the DO concentration in water, and on the liquid	
118	side mass transfer coefficient (k _L a), (Stenstrom and Rosso, 2010; Gostick et al., 1992).	
		5

Oxygen transfer in wastewater is usually affected by the biomass characteristics and the 119 120 design of the aeration system. Aeration and the three parameters that characterize biomass, i.e. particle concentration, particle size and viscosity, are interrelated. Aeration intensity 121 affects particle size and viscosity. Any increase in viscosity has a negative effect on oxygen 122 transfer with the solids concentration modifying it. Oxygen transfer is also affected by the 123 particle size and the particle concentration, whose effects are interrelated. Finally, the 124 125 design of the aeration system additionally affects oxygen transfer with the α -factor being 126 the main parameter that determines the system's aeration capacity. Both β -factor, which 127 accounts for the effect of salts and particulates in wastewater on oxygen transfer, and the 128 temperature-factor have a lesser impact on mass transfer, so they usually need not to be considered. On the other hand, the α -factor indicates the effect of wastewater on oxygen 129 transfer and it varies with wastewater quality, MLSS concentration and the intensity of 130 mixing or the applied turbulence, (Rodríguez et al., 2014, 2012b, 2011, 2010; Germain et 131 al., 2005). 132

PO aeration leads to higher OTEs at lower flowrates under higher organic loadings. As the 133 partial pressure of oxygen in the gas phase increases, the driving force for mass transfer 134 also increases. As such, PO provides higher gas phase oxygen concentrations than air, 135 136 allowing faster treatment under higher MLSS concentrations and shorter HRTs, (Zhuang et al., 2016a; Rodríguez et al., 2013). However, in covered POAS systems in particular, the 137 CO₂ produced may somehow affect oxygen transfer as each time an oxygen bubble is 138 139 inserted into the mixed liquor, CO₂ enters it reducing the partial pressure of oxygen and the saturation concentration of DO, (Gostick et al., 1992). 140 141

In practice, the amount of oxygen that is transferred is highly related to the applied flowrate

- the higher the flowrate, the better the oxygen transfer. For the same flowrate of air and 142

143	PO, the amount of oxygen transferred is higher for the PO due to its higher driving force.
144	PO has a DO saturation concentration that is up to five times higher than that of air at the
145	same flowrate, (Zhuang et al., 2016a; Rodriguez et al., 2013). Lee and Kim (2003), who
146	monitored the DO concentration changes for PO flowrates of 0.0125 L min ⁻¹ , 0.025 L min ⁻
147	¹ , 0.05 L min ⁻¹ , 0.1 L min ⁻¹ and 0.2 L min ⁻¹ , in a tank of a working volume of 21 L, found
148	that when the flowrate changed from 0.0125 L min ⁻¹ to 0.025 L min ⁻¹ , a significant sudden
149	increase in the OTR occurred. For the next flowrates, the DO concentrations continued
150	increasing but more gradually. Similarly, Zhuang et al. (2016a) noticed that, when a PO
151	flowrate of 0.5 L h^{-1} increased to 1.5 L h^{-1} , the DO concentration at a specific time of 300 s
152	was 1.8 times higher. In addition, by comparing PO flowrates with air ones, they also found
153	that at a PO flowrate of 2.5 L h ⁻¹ the DO concentration was five times higher than that for a
154	similar air flowrate. However, there was a limit regarding the DO amount that could be
154 155	similar air flowrate. However, there was a limit regarding the DO amount that could be sustained, so any further increase in the PO flowrate could not increase the OTE value
155	sustained, so any further increase in the PO flowrate could not increase the OTE value
155 156	sustained, so any further increase in the PO flowrate could not increase the OTE value further.
155 156 157	sustained, so any further increase in the PO flowrate could not increase the OTE value further. With regard to the α -factors, Rodríguez et al. (2014), who determined the α -factors in an
155 156 157 158	sustained, so any further increase in the PO flowrate could not increase the OTE value further. With regard to the α -factors, Rodríguez et al. (2014), who determined the α -factors in an MBR fed either with PO or air found that, at a constant HRT, the α -factor increased when
155 156 157 158 159	sustained, so any further increase in the PO flowrate could not increase the OTE value further. With regard to the α -factors, Rodríguez et al. (2014), who determined the α -factors in an MBR fed either with PO or air found that, at a constant HRT, the α -factor increased when MLSS concentrations decreased but, at the same MLSS concentration, the PO related α -
155 156 157 158 159 160	sustained, so any further increase in the PO flowrate could not increase the OTE value further. With regard to the α -factors, Rodríguez et al. (2014), who determined the α -factors in an MBR fed either with PO or air found that, at a constant HRT, the α -factor increased when MLSS concentrations decreased but, at the same MLSS concentration, the PO related α - factor values were higher. Similarly, Rodriguez et al. (2011) showed that, at a constant
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165 MBR even at high MLSS concentrations. In practice, the effect of MLSS concentrations

166 must be determined in combination with the applied HRT, as HRTs regulate the time

167	period during which oxygen is in contact with the liquid. Rodríguez et al. (2012b) showed
168	that the α -factor was highly affected by both MLSS concentrations and the HRTs, but the
169	MLSS concentration was better correlated with the α -factor than the HRT. However, any
170	data based on a specific case study is unable to be directly compared with any other data, as
171	the α -factor strongly depends on the bubble size, the system's geometry and the EPS/SMP
172	concentrations, although the EPS/SMP concentrations have a lesser effect. In addition, α -
173	factors, which can sustain aerobic conditions at bench scale, must be treated with caution
174	when full scale applications are to be considered, (Rodríguez et al., 2012b).
175	In addition, it must be mentioned that Zhuang et al. (2016b) also found that the presence of
176	long chain and aromatic hydrocarbons, as well as of ester compounds, in their coal
177	gasification wastewater, hindered oxygen transfer from the bubbles to the liquid in an MBR
178	fed with air. This consequently led to low DO values in this MBR with the ability of
179	microorganisms to biodegrade having been affected.
180	As a final comment, Rodríguez et al. (2013) also showed that less energy was consumed
181	when PO is employed — the values for oxygen transfer as a function of energy supplied
182	were 0.62 kg ₀₂ h^{-1} kW ⁻¹ for PO and 3.31 kg ₀₂ h^{-1} kW ⁻¹ for air. Previously, Oackley (1997)
183	had similarly mentioned that the power related costs were 25% lower for PO. However,
184	oxygen production entails an additional cost, which is not applicable when air is used.
185	
186	2.2. The Effect of Bubble Size
187	
188	The efficiency of the oxygenation process depends on the oxygen transfer from the bubbly

to the dissolved phase with the total DO in a vessel being closely related to the bubble

190 motion and the hydrodynamic pattern of the fluid flow, (Torti et al., 2013). Oxygen transfer

191 also depends on the interfacial area across which it occurs. Factors, such as the bubble size and the bubble residence time, also have an effect on oxygen transfer, (Gostick et al., 192 1992). Fine bubbles or microbubbles are preferable due to their small size, large interfacial 193 194 area, long stagnation time and lower bubble rising speed as well as they lead to better k_{La} values than usual air or PO bubbles. These bubbles also deal with higher loadings and, at 195 the same time, they form less foam, (Zhuang et al., 2016a,b). Zhuang et al. (2016b) 196 197 employed MBR technology under either air or PO conditions using both usual and fine bubbles and they found that air demonstrated a worse performance. Usual bubbles, made 198 199 either of air or of PO, had also a worse performance on oxygen transfer, although usual PO 200 bubbles still performed better. However, there was also a dramatic increase in the k_La, when they switched from usual PO bubbles to fine PO bubbles. When fine bubbles were 201 used, the gas liquid interfacial area significantly increased due to the decrease in bubble 202 size, (Zhuang et al., 2016b). Coarse bubbles though, may be more efficient in stripping CO_2 203 out of a POAS system reducing the need for use of sodium hydroxide solutions, (Gostick et 204 al., 1992). 205

206

207 **3. EFFECT OF PURE OXYGEN ON BIOMASS**

208

3.1. EPS and SMP Production

210

211 The type of aeration in aerobic WWT affects the extent of production of extracellular

212 polymeric substances (EPS). In batch experiments, EPS concentrations are lower when PO

- is used, (Zhang et al., 2019). Zhang et al. (2019) found that at low food to microorganism
- 214 (F/M) ratios, e.g. 0.05 kg_{TOC} kg⁻¹_{MLSS} (100 mg L⁻¹ TOC and 2000 mg L⁻¹ MLSS), EPS

215	production remained the same regardless of the type of aeration. In both cases, EPS initially
216	increased reaching a peak value and subsequently decreased. This trend is expected as EPS
217	concentration during the substrate utilization phase increases, whereas during endogenous
218	respiration, it starts decreasing as EPS now function as substrate. At the higher F/M ratio of
219	0.25 kg _{TOC} kg ⁻¹ _{MLSS} (500 mg L ⁻¹ TOC, 2000 mg L ⁻¹ MLSS), they observed a similar
220	increase/decrease trend, but the EPS amount in the bioreactor fed with air was still high at
221	the end of the batch reaction due to the high amount of total organic carbon (TOC) that was
222	still present in wastewater.
223	In general, Zhang et al. (2019), claimed that PO use had an enhanced effect on EPS
224	reduction regardless of the initial TOC concentration as well as it better promoted EPS
225	consumption, when that was required. By maintaining the substrate concentration constant
226	at 500 mg _{TOC} L^{-1} and by varying MLSS concentrations from 2000 mg L^{-1} to 5000 mg L^{-1} to
227	8000 mg L ⁻¹ , EPS concentrations initially increased once again and then decreased, except
228	for the highest F/M ratio as applied within the bioreactor fed with air, where a constant
229	increase occurred. EPS rapidly decrease when the growth of microorganisms moves from
230	the exponential increase to the stationary phase where food starts depleting, a case that was
231	never achieved in the air related bioreactor at the highest F/M ratio. As up to 50% of the
232	produced EPS are utilized by bacteria, PO aeration not only accelerated substrate
233	decomposition and increased organic pollutant removal rates, but also promoted
234	decomposition of the produced EPS when that was required, (Zhang et al., 2019).
235	In addition, EPS production is also affected by the applied air or PO flowrates — Zhuang et
236	al. (2016b) found that the higher air flowrates that need to be applied and consequently the
237	higher shear forces in an MBR fed with air produced more EPS, as flocs that are exposed to
238	high shear forces easily break. On the other hand, the lower PO flowrates, in combination

with the enhanced enzyme activity due to the higher DO concentrations, resulted in less

240 EPS. This occurred either because of a lower EPS production or because of an improved

241 EPS biodegradation. Pressurization, which, as already mentioned above is another way of

242 increasing DO concentration in tanks, also lowered EPS production in tanks fed with PO,

243 (Zhang et al., 2019), confirming the important role of DO on EPS formation.

With respect to aerobic granulation, on the other hand, PO aeration may potentially produce 244 245 more EPS than air. In this case, EPS are an important ingredient for stable granules as they ensure their integrity, (Pan et al., 2017). Pan et al. (2017) showed that at 4.57 kg_{COD} m⁻³ d⁻¹, 246 247 the maximum chemical oxygen demand (COD) loading rate they applied, their PO granular bioreactor had the highest amount of EPS that was equal to 193 mg g⁻¹. These EPS were 248 more than those in their PO activated sludge bioreactor, which were more than those in the 249 air granular bioreactor, which were more than those in the activated sludge bioreactor. 250 Under high organic (COD) loadings, biomass grows rapidly. Positive correlations between 251 biomass and EPS and between influent COD and EPS and vice versa were found, as they 252 both increase EPS. PO formed aerobic granules with some great ability of treating heavily 253 polluted wastewaters. This was due to the retention of biomass on an EPS matrix that 254 helped granules avoid disintegration. These granules were difficult to collapse allowing PO 255 256 passing through them, so they tolerated any shocks due to the high organic loadings. 257 In addition, EPS production in aerobic systems is also affected by the salinity, so the combined effect of the aeration type and salinity needs to be considered. As salinity 258 259 increases, the specific oxygen uptake rate (SOUR) also increases, with microorganisms 260 needing more energy to endure the high saline conditions. In saline wastewaters, the large mass transfer resistance makes it difficult for oxygen to diffuse from the gas interface to the 261 cell membrane as well as salinity itself also decreases oxygen solubility. CAS systems 262

cannot cope with the increased DO concentrations required, in particular when high 263 264 loadings are to be treated. PO use instead of air is then a promising alternative, (Hu et al., 2019). Hu et al. (2019) found that, in sequencing batch reactors (SBRs), PO improved the 265 266 TOC removal efficiency at salinities less than 3%. Increasing salinity to values over 3.5%, the TOC removal efficiency decreased regardless of the type of aeration, however, PO still 267 performed better than air. As salinity kept increasing, even high DO concentrations had a 268 269 negligible effect on metabolism, because of the strong shock on the bacteria. EPS under 270 either type of aeration increased as salinity increased, with PO mostly producing more EPS 271 than air at the same salinity. In detail, at low salinities up to 1%, for both aeration types, 272 EPS amounts were similar. For salinities above 2%, EPS at PO aeration conditions increased. PO mitigated the effect of high viscosity on oxygen transfer and promoted EPS 273 production as a measure to protect the microorganisms from salt suppression. At a salinity 274 of 3%, polysaccharides increased over time in both aeration types, but not the proteins. 275 Initial concentrations of polysaccharides in both systems were lower than those of proteins, 276 but as salinity increased, their concentration exceeded the concentration of the proteins, 277 which remained stable. The production of polysaccharides was then the bacterial reaction 278 against the high osmotic pressure due to salt. As such, their concentration was higher when 279 280 PO was used due to the higher partial pressure, (Hu et al., 2019). 281 In addition, Hu et al. (2019) found that the soluble microbial products (SMP) also increased 282 under both aeration types, as salinity increased. At the highest salinity of 5%, SMP were maximum in both bioreactors — $63.28 \text{ mg } \text{L}^{-1}$ (PO) and $62.5 \text{ mg } \text{L}^{-1}$ (air). At low salinities 283 of 0.5% or 1%, SMP in the bioreactor fed with PO were more due to the sufficient 284 degradation rate. At salinities of 2%, 3% or 4%, the opposite happened. The increased EPS 285 production to help bacteria tolerate the saline conditions reduced the production rate of 286

287	biomass associated SMP in the bioreactor led with PO. At a samily of 5%, the effect of
288	salinity was dominant, as high osmotic pressures started rupturing the cells leading to
289	higher biomass associated SMP production rates due to the EPS hydrolysis at a rate that
290	was proportional to the EPS production.
291	Finally, by running batch experiments, Zhang et al. (2019) showed that at a low F/M ratio
292	of 100 mg _{TOC} L ⁻¹ , SMP concentration remained stable regardless of the type of aeration. On
293	the other hand, at a higher F/M ratio of 500 mg $_{TOC}$ L ⁻¹ , SMP concentrations were always
294	higher when PO was used. Improved oxygen transfer improved the enzyme and biomass
295	activity, so the production of utilization associated SMP at high F/M ratios increased.
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3.2. Enzyme Activity and Microbial Diversity

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The enzyme activity shows the ability of bacteria to adapt themselves to environmental 299 changes. During the formation of activated sludge, microorganisms use their enzymes, e.g. 300 catalase, dehydrogenase, phosphatase, protease, esterase, glucosidase, to hydrolyze and 301 biodegrade organic matter, mostly consisting of proteins and carbohydrates. Based on their 302 variations, the physiology of the bacterial community is assessed, (Calderón et al., 2012). 303 304 Enhanced enzyme activity leads to better multiplication conditions for the living 305 microorganisms and subsequently improves the pollutant removal efficiency. However, at low DO conditions and high organic loading rates (OLRs), the enzyme activity deteriorates. 306 307 PO use may then accelerate it, so it will consequently improve the microbial biomass activity as well — the increased substrate utilization rates when PO is used are interrelated 308 309 with higher enzyme activity. PO aeration demonstrated higher concentrations for many of the enzymes. Microorganisms can adapt to PO aerated environments, so that the secretion 310

| 311 | of enzymes stabilizes at high values, (Pan et al., 2017; Zhuang et al., 2016a; Doviral-García                      |
|-----|--------------------------------------------------------------------------------------------------------------------|
| 312 | et al., 2014). In addition, Zhuang et al. (2016b), by measuring SOURs, showed that when                            |
| 313 | PO was used, SOUR was 4.15 $mg_{O2} g_{MLSS}$ <sup>-1</sup> h <sup>-1</sup> , or 28% higher than the SOUR that was |
| 314 | measured when air was used.                                                                                        |
| 315 | However, the improved biomass activity in POAS may also have an adverse effect, which                              |
| 316 | needs considering, as it results in rapid DO depletion in the secondary clarifiers, which                          |
| 317 | usually lack any aeration facility — a condition which can be additionally favored under                           |
| 318 | higher ambient temperatures, as these temperatures further improve bacterial metabolism.                           |
| 319 | In more detail, to avoid septic sludge and poor separation of solids from liquid as well as                        |
| 320 | foul odours, DO levels must be always maintained at the required level. As the mixed                               |
| 321 | liquor approaches the last stage in the aeration tank, DO concentrations drop and CO <sub>2</sub>                  |
| 322 | concentrations increase. Increased CO <sub>2</sub> levels decrease the pH promoting the growth of                  |
| 323 | filaments (that also thrive at low substrate concentrations) and fungi, which consequently                         |
| 324 | hinder sludge settling and compaction of sludge. Sludge bulking will finally increase the                          |
| 325 | concentrations of total suspended solids (TSS) in the effluent and cause losses of active                          |
| 326 | biomass. In case oxygen becomes the limiting substrate, glucose consumption rates also                             |
| 327 | increase causing sludge deflocculation. Therefore, sludge exhibits poor settling properties                        |
| 328 | in the secondary clarifiers resulting in effluents of deteriorated quality. Any use of the                         |
| 329 | design of and of the operational guidelines for CAS systems to POAS systems is not                                 |
| 330 | advisable as their microbial populations and the metabolic rates may not be similar. The                           |
| 331 | challenge above must then be considered when POAS systems are to be designed, (Kundral                             |
| 332 | et al., 2013; Lee et al., 2003).                                                                                   |
| 333 | Gostick et al. (1992) had also observed that at low F/M ratios, the POAS plant under their                         |
|     |                                                                                                                    |

334 study, which was treating a vegetable processing wastewater, confronted bulking issues, as

335 filaments growth was higher than that of floc forming bacteria. Low F/M ratios may indeed lead to the production of filaments and poorly flocculated pin flocs due to aged sludge, 336 (Paice et al., 2003; Marshall and Sousley, 1997). On the other hand, too high F/M ratios 337 338 may lead to dispersed growth, (Paice et al., 2003). Finally, Calderón et al. (2013), by comparing the effect of the type of aeration on the 339 performance of hydrolytic enzymes, did not detect any difference. Any increase in the 340 341 pollutant removal efficiency during PO aeration was found to be unrelated to any improvement of the depolymerization of the particulate matter, (Calderón et al., 2013). This 342 343 field does require further research, as some enzymes are highly affected by the redox of a 344 system, whereas others, like protease and esterase, are not, despite being very important for the hydrolysis of macromolecules and contaminants, (Doviral-García et al., 2014). 345 The aeration type may also affect the microbial diversity, as the former can promote 346 different species composition of activated sludge communities, (Zhuang et al., 2016a,b). 347 Zhuang et al. (2016b) claimed that the bacterial communities are not the same in an MBR 348 349 fed with air and an MBR fed with PO. First, based on the ten most abundant genera present in MBRs, their total relevant abundance in the MBR fed with PO was found to be higher 350 than that in the MBR fed with air — 35.35% and 28.03% respectively. The MBR fed with 351 352 PO concentrated more genera in a small fraction, indicating their adaption to higher DO concentrations. Then, although both MBRs did share a large proportion of core bacterial 353 population, some differences were also observed, i.e. concentrations of Ohtaekwangia, 354 355 which are detected during coal mine wastewater treatment, Thauera, which degrade phenol and methyl-phenols and Comamonas, an aromatic compounds degrader, increased in the 356 MBR fed with PO, explaining also the better effluent quality. Finally, increased amounts of 357 Phycisphaera in the MBR fed with PO additionally demonstrated its potential for nitrogen 358

removal, (Zhuang et al., 2016b). However, when Calderón et al. (2012, 2013) compared MBRs employing PO with MBRs employing air, they concluded that the aeration type had only a negligible effect on the diversity and functionality of the bacterial community, which was particularly true when this effect had to be compared with the effects of temperature or VSS concentration. However, they also supported the fact that that type of aeration affected the bacterial community structure and differences in the relative abundance of dominant populations were once again recorded, (Calderón et al., 2012, 2013).

366

- **367 3.3. Foam and Froth Formation**
- 368

Excessive foam formation can affect the final water quality, as bacteria trapped in foam die, 369 so the aerobic treatment performance and the system's operational stability deteriorate. In 370 the case of air, the large amounts of air that have to be used for sufficient DO 371 concentrations cause foam formation, which hinders digestion and promotes biomass 372 washout. To avoid or control this, as low flowrates as possible have to be applied, but these 373 flowrates may deprive bacteria of the required DO. The use of PO may once again be help, 374 (Zhuang et al., 2016a,b; Zupančič and M. Roš, 2008; Lee and Kim, 2003). Zhuang et al. 375 376 (2016a,b), concluded that the improved removal efficiency in MBRs fed with PO was attributed to the prevention of foam expansion. Zhuang et al. (2016a) achieved DO 377 concentrations of 10 mg  $L^{-1}$ , with a foam to liquid (F/L) ratio of 6%-10% for their MBR 378 379 that was fed with PO and of 30%-45% for the MBR that was fed with air. Since air is only 21% O<sub>2</sub> in volume, the air flowrate needed to be up to five times higher leading to 380 excessive foam formation, (Zhuang et al., 2016a,b). 381

382 CAS applications also suffer from froth caused by Nocardioform organisms. These aerobic gram positive hydrophobic filaments preferentially concentrate at the air liquid surface and 383 produce thick viscous froth in both the aeration basins and the secondary clarifiers. Froth 384 385 causes a series of problems, related to either the liquid itself or the solids handling including deterioration of effluent quality. Both CAS installations, particularly those 386 operated at long SRTs, and POAS installations are equally affected by froth. POAS plants 387 388 are affected by froth due to them containing significant surface trapping of activated sludge. One way of avoiding froth in POAS proposed Jolis et al. (2006) was the application of a 389 390 low SRT and selective wasting. They found that an SRT of 0.3 days resulted in complete 391 removal of filamentous microorganisms in two days, which subsequently allowed the POAS plant to operate successfully at an SRT up to 3 days without confronting any return 392 of the filaments. In a further analysis, Jolis et al. (2007) additionally highlighted the 393 importance of using anaerobic selectors in POAS to promote growth of phosphorous 394 395 accumulating organisms (PAOs), which outcompete filaments. Under operating conditions 396 favoring enhanced biological phosphorous removal (EBPR), that was at an SRT<2 d and an HRT>55 min, a decrease in filamentous organisms occurred resulting in effective froth 397 control as well, (Jolis et al., 2007). 398

To this point, it may also be worth mentioning that the use of PO, on the other hand, may also be able to stimulate the growth of PAOs in EBPR systems, which alternate anaerobic and aerobic environments. Wei et al. (2014), who operated laboratory scale pressurized oxygen aeration SBRs, found that PO increased the oxygen-reduction potential (ORP) from the highly negative value required during the anaerobic treatment to the positive value required for the production of the oxidative environment for phosphorous uptake but no more details were given.

# **3.4. Pure Oxygen Aeration and Temperature**

| 408                      | Special mention should be made on the effect of the type of aeration in combination with                                                                                                                                                                                                                                                                                      |
|--------------------------|-------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 409                      | temperature. Temperature is an operating parameter that heavily affects bacterial                                                                                                                                                                                                                                                                                             |
| 410                      | metabolism, so its effect on biomass is more dominant than the type of aeration — any                                                                                                                                                                                                                                                                                         |
| 411                      | temperature changes may lead to poor sludge settling, high turbidity of the final effluent,                                                                                                                                                                                                                                                                                   |
| 412                      | etc., (Rodríguez et al., 2014; Bernat et al., 2017). However, Rodríguez et al., (2011),                                                                                                                                                                                                                                                                                       |
| 413                      | observed that in an MBR fed with PO operating at an SRT of 39.91 days, the 0.4 $g_{SS}$ g <sup>-1</sup> <sub>COD</sub>                                                                                                                                                                                                                                                        |
| 414                      | of sludge that was produced was similar to those reported in other studies, where air had                                                                                                                                                                                                                                                                                     |
| 415                      | had been used, however, in their study that was achieved at a lower temperature. As                                                                                                                                                                                                                                                                                           |
| 416                      | temperature affects sludge production with the latter decreasing as the former increases,                                                                                                                                                                                                                                                                                     |
| 417                      | their improved value can be attributed to the PO, which maintained sludge production at                                                                                                                                                                                                                                                                                       |
| 418                      | low levels.                                                                                                                                                                                                                                                                                                                                                                   |
| 419                      | A case of interest though is the aerobic thermophilic process, which is carried out at                                                                                                                                                                                                                                                                                        |
| 420                      | temperatures higher than 45°C. Comparer to mesophilic processes, it leads to higher                                                                                                                                                                                                                                                                                           |
|                          |                                                                                                                                                                                                                                                                                                                                                                               |
| 421                      | biodegradation rates, inactivation of pathogens and lower excess sludge production, but to                                                                                                                                                                                                                                                                                    |
| 421<br>422               | biodegradation rates, inactivation of pathogens and lower excess sludge production, but to effluent of poorer quality. Effluent's higher COD and turbidity are due to the large amount                                                                                                                                                                                        |
|                          |                                                                                                                                                                                                                                                                                                                                                                               |
| 422                      | effluent of poorer quality. Effluent's higher COD and turbidity are due to the large amount                                                                                                                                                                                                                                                                                   |
| 422<br>423               | effluent of poorer quality. Effluent's higher COD and turbidity are due to the large amount<br>of dispersed free bacteria and colloids, which hinder thermophilic sludge to settle in                                                                                                                                                                                         |
| 422<br>423<br>424        | effluent of poorer quality. Effluent's higher COD and turbidity are due to the large amount<br>of dispersed free bacteria and colloids, which hinder thermophilic sludge to settle in<br>secondary clarifiers, (Collivignarelli et al., 2015). Indeed, Cohen (1977) had already found                                                                                         |
| 422<br>423<br>424<br>425 | effluent of poorer quality. Effluent's higher COD and turbidity are due to the large amount<br>of dispersed free bacteria and colloids, which hinder thermophilic sludge to settle in<br>secondary clarifiers, (Collivignarelli et al., 2015). Indeed, Cohen (1977) had already found<br>that high biomass reduction in an uncovered POAS system had been achieved due to the |

429 aerobic step, either PO or air had to be used. However, PO aeration and high thermophilic430 temperatures were found not to be compatible.

Aerobic sludge digestion, or the extension of CAS process under endogenous respiration 431 conditions, requires a lot of oxygen or the process is disturbed. In the thermophilic range, 432 permanent lack of DO is monitored in excess sludge, as the potential of oxygen for 433 absorption is low due to poorer solubility. In addition, the oxygen demand is higher due to 434 435 much higher rates of sludge digestion. Even though the mixing of sludge water, that is to 436 say any water remaining after sludge digestion, with the main feed, does not increase the 437 COD in the feed, it does contribute to as much as 50% of the total ammonium, (Zupančič 438 and Roš, 2008). Sludge treated with PO by Zupančič and Roš (2008) degraded between 22°C and 50°C, whereas sludge treated with air degraded between 32°C and 65°C. When PO 439 was employed, no sludge digestion took place above  $50^{\circ}C$  — in such temperatures, such 440 high DO concentrations do not occur in natural environments, so bacteria were unlikely to 441 442 tolerate them. On the other hand, in the mesophilic range, the PO had a better performance. Zupančič and Roš (2008) showed that both types of aeration had both advantages and 443 disadvantages, with temperature prevailing against the aeration type. High temperatures 444 promoted better digestion with air, something that was impossible for the PO, which at 445 446 lower temperatures performed better. Finally, Collivignarelli et al. (2015), by performing ammonia utilization rate tests at 49°C using thermophilic biomass taken from a bioreactor 447 fed with PO, showed that low nitrification rates in the range of  $<0.01 \text{ mg}_{N-NO3}^{-1} \text{ gr}_{VSS}^{-1} \text{ h}^{-1}$ 448 449 were obtained, so no biological oxidation of ammonium through nitrification occurred. This also supports the need for lower temperatures during PO use. 450

451

452

#### 453 **4. EFFECT OF PURE OXYGEN ON EFFLUENT QUALITY**

454

#### 455 **4.1. Organic Carbon Removal**

456

457 PO aeration achieves high pollutant removal efficiencies at low oxygen flowrates. Table 3
458 shows the pollutant removal efficiencies of selected studies, which are high, including those
459 of organic carbon.

460

461 TABLE 3: Pollutant removal efficiencies as indicated in a number of selected studies462

However, a proper direct comparison of the relative performance of the CAS and POAS 463 has long since been not an easy task as it needs involvement of many factors and requires 464 complex experimental designs, (Cohen, 1977; Dirk, 1981). Any chance for advanced 465 performance of POAS had to be solely attributed to the higher partial pressures and its 466 ability to transfer oxygen rapidly and not on any significant differences in the intrinsic 467 kinetic parameters and settling characteristics, (Shelef and Green, 1980). Rempel et al. 468 (1992), by operating a pilot CAS and a pilot POAS, treating mill effluent at different sets of 469 470 F/M ratios, SRTs and HRTs, showed that under similar sets of operating conditions, 471 biochemical oxygen demand (BOD) and COD removal efficiencies between the two systems were not very different. They also pointed that the effect of operating times on the 472 473 removal efficiencies was more important than that of the type of aeration. Esparza-Soto et al. (2006a) additionally claimed that although PO increases treatment 474 capacity of the aeration stage and produces biosolids that settle better, its benefits against 475 air are still under discussion. PO systems, at high F/M ratios, are very successful in 476

477 removing five-day BOD (BOD<sub>5</sub>) and suspended solids (SS) as well as in producing little sludge. In addition, COD removal efficiencies in these systems also remain high, about 478 80% for F/M ratios up to 2.8 mg<sub>BOD5</sub>/mg<sub>MLVSS</sub>. However, this COD removal efficiency was 479 480 also the same when air was used as well. The difference was that in the case of air, the range of F/M ratios applied was shorter and equal to only one third of that one reached by 481 PO. For F/M ratios up to about 1.25 mg<sub>BOD5</sub>/mg<sub>MLVSS</sub> or 1.5 mg<sub>COD</sub>/mg<sub>MLVSS</sub>, both aeration 482 483 types remove similar COD amounts. Nonetheless, at higher F/M ratios, a fair comparison cannot be made, as the systems fed with air have never been run at such high F/M ratios 484 485 due to their restricted oxygen transfer capacity. 486 In addition, Esparza-Soto et al. (2006b) determined the molecular weight (MW) distribution in wastewater samples from full-scale WWT plants using different aerobic 487 treatments and concluded that the plants generated effluent organic matter (EtOM) with 488 different MW dissolved organic carbon (DOC) distributions. Systems operating with air 489 490 generated E<sub>f</sub>OM with centrally clustered distribution — the intermediate MW fraction contained most of the organic matter, i.e. 50%-60%, DOC: 0.5 kDa to 3 kDa. Systems 491 operating with PO, on the other hand, generated E<sub>f</sub>OMs with skewed distributions towards 492 the high MW fraction, i.e. 40%-50%. Long SRTs reduced the DOC concentration in the 493 494 effluent, but the MW distribution remained unmodified. PO generated organic matter of a more refractory character. That was due to the higher DO concentrations, which increased 495 the endogenous respiration rate and promoted the production of EPS and SMP, which are 496 497 high MW refractory biopolymers. Both aeration types were then equally efficient up to certain F/M ratios, although PO systems extended operation to higher F/M loadings. 498 However, if we aim for the production of EfOM of better quality — less DOC with lower 499

500 MW — the systems operating with air were more efficient within the range of F/M ratios where the use of both air and PO was applicable, (Esparza-Soto et al., 2006a,b). 501 On the other hand, there are also studies which clearly state that the use of PO improves 502 effluent quality. For example, Zhang et al., 2019, found different TOC biodegradation 503 504 levels for PO and air, with PO showing a better performance. In addition, an increase in the 505 MLSS concentration increased the gap between the PO and air related removal efficiencies, from 16.8% of TOC removal at 2000 mg  $L^{-1}$  to 76.5% at 8000 mg  $L^{-1}$ . Pan et al. (2017) 506 also observed more filamentous, actinophryids and nematodes when air was used as well as 507 508 the percentage of the total aerobic bacteria was higher when PO was used. However, in 509 both cases, they carried out batch tests at small scale and, as PO acts faster than air, its use may be more advantageous with respect to operation restricted to short times. Finally, 510 Bernat et al. (2017) found that despite the abundance of Vorticella infusionum, whose 511 presence indicates unfavorable treatment conditions, in the mixed liquor, a stable and 512 acceptable effluent quality was achieved in their POAS system, but this is more related to 513 the fact that PO manages to better maintain aerobic conditions. As a conclusion, either 514 under air or PO conditions, there seems to be no significant difference in terms of organic 515 carbon removal. However, PO may be of help in cases where operational parameters do not 516 517 support the use of air.

518

#### 519 4.2. Removal of Phenolic Compounds and Micropollutants

520

521 Regarding phenolic compounds, PO use is quite promising. Moerman et al. (1995), by

522 operating a POAS plant treating pretreated diluted carbonization wastewater, proved that

523 98% of thiocyanate, a substance whose overloading inhibits any phenol degradation, had

524 been removed. In addition, Li and Loh (2006) found that during the cometabolism of 4chlorophenol in the presence of phenol in an immobilized cell hollow fibre MBR enriched 525 with Pseudomonas putida, both 4-chlorophenol and phenol degradations improved when 526 PO was used. Finally, PO was satisfactorily used in continuous flow fluidized bed reactors 527 528 for the degradation of polychlorinated phenols that are included in wood preservation chemicals, (Puhakka and Järvinen, 1992). 529 530 In addition, PO use can also help with the removal of refractory pollutants and 531 micropollutants. CAS cannot remove antibiotics, endocrine disrupting compounds, 532 pharmaceuticals and residual personal care products, household and industrial chemicals, 533 etc. as CAS plants have not been designed or operated for this purpose, (Batt et al., 2007). Levine et al. (2006) assessed the persistence of a number of micropollutants during primary 534 treatment, biological treatment comprising POAS and nitrification/denitrification and 535 finally disinfection. Several substances tested had lower concentrations in the denitrified 536 effluent than in the influent or the primary treatment effluent, which means that biological 537 treatment with PO additionally helped with their removal. For instance, acetaminophen, a 538 non-antibiotic over-the-counter pharmaceutical that had been detected at the highest 539 concentration of 10 µg L<sup>-1</sup> in the influent, was eliminated during biological treatment. 540 541 Batt et al. (2007) monitored the fate of four antibiotics, i.e. ciprofloxacin (CIP), sulfamethoxazole (SMX), tetracycline (TC) and trimethoprim (TRI), in four full scale 542 WWT plants including POAS. The use of PO had a positive effect on the removal of the 543 544 antibiotics, mainly with respect to removal of SMX and TRI, despite the short HRT of 1 h that was applied. In all plants tested, removal efficiencies were strongly related to the 545 operating times. However, by comparing the performance of the POAS plant with that one 546 of the rotating biological contactor (RBC) plant, it was found that, although similar 547

removal efficiencies for all antibiotics were reported for both plants, the HRT at POAS wasfour time less than that at RBC.

Bae et al. (2015) also assessed the contribution of PO aeration of a combined biological and 550 551 physicochemical treatment (POAS + Fenton Process) system to the removal of refractory pollutants from dyeing wastewater, which is not a readily biodegradable wastewater as well 552 as any potential improvement with respect to biological treatment of dyes needs long SRTs 553 554 and high MLSS concentrations. Although the biological treatment system suffered from low MLSS concentrations due to inefficient settling, it managed to remove 53% of soluble 555 556 COD (SCOD) and 12% of color indicating that the microorganisms in the aeration tank 557 were acclimated to the dyeing wastewater. As such, Bae et al. (2015) managed to obtain a cost effective pretreatment, which both helped the Fenton Process in becoming more 558 efficient and decreased the consumption of chemicals. 559 Martín-Rilo et al. (2018) employed PO aeration for the removal of a benzotriazole based 560 anticorrosive from dairy wastewater with benzotriazole being an aromatic compound used 561 as a metal corrosion inhibitor and an emerging toxic that tends to bioaccumulate. PO was 562 injected in the intermediate step (Step 2) of a treatment process — that step was preceded 563 by wastewater neutralization under CO<sub>2</sub> injection (Step 1) and followed by 564 565 coagulation/flocculation in a dissolved air floatation tank (Step 3). Steps 1, 2, and 3 removed 44%, 30%, and 25% of the total benzotriazole respectively for an overall removal 566 of 99.7%. The respective removal efficiencies of each step considering the concentration of 567 568 the contaminant entering each step was 44%, 53%, and 95.6% respectively, so more than 569 half of the amount of the chemical that entered Step 2 was removed. Finally, PO aeration may also be beneficial regarding the removal of endocrine disruptive 570

571 compounds from wastewater. For example, bisphenol-A, the endocrine disrupting chemical

| 572 | of the greatest concern, due to its effects being more detrimental than those of other        |
|-----|-----------------------------------------------------------------------------------------------|
| 573 | substances of the same kind, cannot be removed by CAS. In addition, its removal does not      |
| 574 | necessarily guarantee its complete degradation, as, due to its hydrophobic nature, it is also |
| 575 | be stored in the sludge or, where possible, it can be adsorbed by membranes, (Doviral-        |
| 576 | García et al., 2014). Dorival-García et al. (2014) assessed then the fate of bisphenol-A by   |
| 577 | employing MBR technology to prolong SRT, and they found that bisphenol-A reached the          |
| 578 | background level in the effluent in five days when PO was used, whereas it took 10 days in    |
| 579 | the case of air. By switching from air to PO, they also improved the biodegradation           |
| 580 | percentages from 60.2% to 87.4% in 10 days. With air, bisphenol-A that was sorbed in the      |
| 581 | sludge, remained within it, so its availability for biodegradation was reduced and the need   |
| 582 | for further sludge treatment before sludge disposal in order to remove the sorbed             |
| 583 | contaminant increased. On the other hand, with PO, the sorption tendency diminished and       |
| 584 | desorption was facilitated — 67.4% of bisphenol-A in the sludge underwent desorption and      |
| 585 | consequently biodegradation.                                                                  |
|     |                                                                                               |

## 587 4.3. Removal of Volatile Organic Compounds

588

PO aeration is also highly capable of handling foul condensates without stripping or of biodegrading VOCs. To this end, Freitas dos Santos and Livingston, (1993a,b) proposed a gas enclosed recirculation system based on a bioreactor fed with PO, whose design was similar to that of a an air lift bioreactor, for the aerobic degradation of the 1,2dichloroethane in 1,2-dichoroethane contaminated wastewater. Air stripping of 1.2dichloroethane that usually takes places during its aerobic treatment was avoided — VOC emissions are known to cause immediate toxicity and odor and promote undesired chemical

| 596 | reactions. In their conventional air lift bioreactor, 33% of the 1,2-dichloroethane was lost  |
|-----|-----------------------------------------------------------------------------------------------|
| 597 | due to stripping, whereas in the system where PO was used, the whole of 1.2-                  |
| 598 | dichloroethane was mineralized.                                                               |
| 599 | In addition, Paice et al. (2003) mentioned that one of the selling points of POAS systems     |
| 600 | for kraft pulp mills was their ability to treat their foul condensates without any stripping, |
| 601 | even from the start-up. However, these systems were liable to some problems, which were       |
| 602 | mainly directly related to their enclosed design. As such, the potential for high             |
| 603 | concentrations of dissolved $CO_2$ in the effluent, the potential for combustible gas alarms  |
| 604 | caused by VOCs in the condensates and possible premature corrosion of the concrete,           |
| 605 | which was associated with the bacteria that reduce sulfuric compounds, had to be carefully    |
| 606 | considered. These problems can also be additionally intensified, as this design further       |
| 607 | complicates the inspection and maintenance of the systems, (Paice et al., 2003).              |
|     |                                                                                               |

#### 609 4.4. Effect of Pure Oxygen on Heavy Metals

610

Heavy metals are non-biodegradable toxic substances that interact with the biomass and 611 have various inhibitory or toxic effects on bacteria, with nitrifying autotrophs being more 612 sensitive to them than heterotrophs, (Avezzù et al., 1995). Avezzù et al. (1995) assessed the 613 fate of heavy metals during treatment of leachates by monitoring their distribution in the 614 solid or liquid phase of the biological process and showed that, despite their accumulation 615 616 in the biomass, BOD<sub>5</sub> and COD removal efficiencies remained quite high. As such, PO aeration created a quite favorable living environment for the microorganisms, which, by 617 making microorganisms more resistant to compounds like heavy metals, managed to 618 maintain considerable removal of organic carbon, (Avezzù et al., 1992). 619

#### 5. PURE OXYGEN AND NITRIFICATION

## **5.1. Introduction**

| 624 | Ammonia that is not removed during WWT can cause a number of adverse environmental                        |
|-----|-----------------------------------------------------------------------------------------------------------|
| 625 | impacts, when effluent is discharged into the receiving water bodies. These include                       |
| 626 | eutrophication, DO depletion and toxicity to aquatic organisms. Removal of nitrogen                       |
| 627 | occurs during nitrification and denitrification. Aerobic autotrophic nitrification is an                  |
| 628 | oxygen demanding process comprising two phases: i) ammonium is oxidized to nitrite,                       |
| 629 | usually by Nitrosomonas, and ii) nitrite is oxidized to nitrate by Nitrobacter. During                    |
| 630 | denitrification, nitrite and nitrate are reduced to nitrogen gas, (Neerackal et al., 2016;                |
| 631 | Rodríguez et al., 2012c). High DO levels reduce the competitive stress upon the                           |
| 632 | autotrophic nitrifying bacteria, with a constant DO concentration of 4 mg L <sup>-1</sup> being high      |
| 633 | enough to achieve nitrification, (Moerman et al., 1995). Nitrification is a complex process               |
| 634 | where PO aeration is evaluated together with other parameters like pH and temperature.                    |
| 635 | Nevertheless, DO concentration is still used to control nitrification, particularly in the cases          |
| 636 | of high ammonia loading rates or low temperatures, (Bonomo et al., 2000). DO                              |
| 637 | concentration also controls nitrification phases as concentrations $<0.5 \text{ mg L}^{-1}$ ensure stable |
| 638 | inhibition of nitrite oxidizing bacteria, (Canziani et al., 2006).                                        |
| 639 |                                                                                                           |
| 640 | 5.2. The effect of pH                                                                                     |
| 641 |                                                                                                           |
| 642 | POAS is more vulnerable to low pH inhibition of nitrification than CAS because of their                   |

643 closed headspace design, whose main role is to minimize oxygen losses by recycling

headspace gas. Because of the aerobic treatment, the headspaces can contain elevated 644 amounts of CO<sub>2</sub>, which under slightly increased pressure moves into the mixed liquor 645 reducing the pH, whose degree of reduction depends on parameters like the system's buffer 646 647 capacity or the degree of venting. This decrease affects the kinetics of enzyme reactions, the bacterial species predominance and the physical properties of the organisms and 648 particles. When nitrification has to take place, pH reduction additionally inhibits it, unless 649 650 acclimation has already taken place. As nitrification proceeds, alkalinity is consumed, 651 which further reduces the pH — entrapped CO<sub>2</sub> does not reduce alkalinity itself but it does 652 reduce the pH at a given alkalinity. Many POAS systems were originally designed only for 653 removal of organic carbon, therefore to operate at low SRTs and HRTs ranging from 1.5 h to 2.5 h. In practice, nitrification could proceed at lower pH values in covered POAS 654 systems, however, this requires contact times up to 3.5 h-5 h, which are rather longer than 655 those usually applied to handle carbon. To resolve this, another process step may then have 656 to be added, (Garber, 1977; Dirk, 1981; Shelef and Green, 1982; Sear et al., 2003). In 657 658 addition, to reinstate nitrification that had been hindered by a pH of 6.5, Mauret et al. (2001) either alternated aeration with air and PO or combined the two. Nitrification that was 659 non-existent at pH values <6 could also have been held if acclimation had preceded, e.g. 660 661 Sears et al. (1995) found that nitrification in POAS was stable at pH values between 5 and 5.5 provided that the required acclimation period had been applied. 662 As combined carbonaceous and nitrogenous removal in POAS systems that are designed to 663 664 remove only organic carbon is difficult, particularly in enclosed ones, Bonomo et al. (2000) proposed as an alternative the use of MBBRs aerated with PO for tertiary nitrification of 665 the secondary effluent. The extra treatment step eliminated any competition between 666 heterotrophic and autotrophic bacteria and PO aeration increased nitrification without 667

| 668 | requiring a certain increase in the thickness of the biofilm. They then managed to acquire                             |
|-----|------------------------------------------------------------------------------------------------------------------------|
| 669 | high nitrification rates both in ammonia and in oxygen limiting conditions. Maximum                                    |
| 670 | efficiencies were seen at lower ammonia loading rates. Efficiencies higher than 90% were                               |
| 671 | also achieved at ammonia loading rates higher than 4 $g_N m^{-3} d^{-1}$ provided that the DO                          |
| 672 | concentration was higher than 10 mg L <sup>-1</sup> -15 mg L <sup>-1</sup> , clearly stating the positive contribution |
| 673 | of the DO use. Finally, alteration of PO use with air use, and vice versa, depending on the                            |
| 674 | pH value, may be able to simultaneously treat carbon and ammonia in one basin and this                                 |
| 675 | seems to be technically feasible, (Mauret et al., 2001).                                                               |

- 676
- 677 **5.3. The Effect of Temperature**
- 678

The effect of temperature on nitrification is complicated, as low temperatures reduce 679 nitrification but increase oxygen's solubility. During nitrification in a POAS system at 12°C, 680 Sears et al. (1995) proved that the HRT did not have any effect on the specific nitrification 681 rates but the temperature controlled the process instead. Increasing the temperature at 24°C, 682 an HRT of 4 h functioned better than that of 2.5 h. By adding an anoxic reactor, the pH 683 increased as the alkalinity managed to recover due to denitrification — i.e. 3.57 mg<sub>CaCO3</sub> 684 685 was produced per mg<sub>NO3-</sub> that was reduced. The anoxic tank itself did benefit nitrification, however, even without it nitrogen loss happens in aerobic systems to some extent — anoxic 686 conditions can occur inside the flocs or due to some bacterial species that can perform 687 denitrification under aerobic conditions, Sears et al., 2003, 1995). Indeed, Neerackal et al. 688 (2016), by using Alcaligenes faecalis strain No. 4, which have the ability to turn 689 ammonium in wastewater to nitrogen in one single aerobic process, showed that within a 690 batch operation of 24 h, total ammonium removal from their dairy wastewater was about 691

692 100% when the reactor's headspace was flushed with PO and 42% when it was flushed693 with air.

Salvetti et al. (2006) also operated MBBRs fed with PO to monitor the combined effect of 694 the temperature and the type of aeration on nitrification. PO diffuses more deeply into the 695 biofilm than air, so it produces higher nitrification rates, hence, requiring smaller reactor 696 volumes, (Salvetti et al. 2006). At low ammonium concentrations, Salvetti et al. (2006) 697 698 found PO was not essential as air could provide the required DO and the temperature did not have any significant effect on the nitrification rates. As ammonium concentration 699 700 increased, DO became the reaction limiting substrate (this occurs even at DO concentrations as high as 5 mg L<sup>-1</sup> or 10 mg L<sup>-1</sup>), so PO use was preferred. These findings 701 are also in line with the findings of Bonomo et al., (2000) mentioned in Section 5.2. Under 702 oxygen limiting conditions, the specific biomass activity, as the ratio of nitrification rate 703 704 to biomass content on the support media, was higher between 23°C-28°C than between 705  $18^{\circ}$ C- $22^{\circ}$ C. By removing the effect of DO on biomass, whose solubility also decreases as 706 temperature increases, that was further attributed to the higher temperature that favors nitrification and to the reduced resistance to diffusion, which allows more biomass to have 707 access to DO. 708

709

#### 710 5.4. The Effect of Operational Times

711

The role of SRT is important for POAS, as longer SRTs favor nitrification, (Sears et al.,

2003). As nitrifiers grow slowly, their growth is strongly related to the applied SRT. Due to

the high treatment rates that are achieved thanks to their high OTRs, POAS plants are

usually operated at short sludge ages ranging from 1 day to 3 days, which are not long

| 716 | enough for nitrification, (Neethling et al., 1998). To overcome this, Neethling et al. (1998)        |
|-----|------------------------------------------------------------------------------------------------------|
| 717 | transferred aged waste activated sludge from a CAS plant to a POAS plant to seed it with             |
| 718 | nitrifiers. This increased the "apparent" sludge age and the high loaded POAS plant                  |
| 719 | achieved nitrification. After the seeding process, they found that ammonium concentration            |
| 720 | in the effluent decreased to 5 mg $L^{-1}$ within 3 days stabilizing to values below 0.5 mg $L^{-1}$ |
| 721 | after about a month. Similarly, Randall and Cokgor (2001) applied lower flows and                    |
| 722 | continuous seeding to maintain nitrification to a full scale POAS system.                            |
| 723 | In addition, Rodriguez et al. (2012c) studied the effect of HRT on nitrification under either        |
| 724 | PO or air conditions by employing MBR technology for the treatment of primary clarifier              |
| 725 | effluent. They showed that, by using PO instead of air, nitrogen removal efficiency was              |
| 726 | improved by 8% at an HRT of 12 h and by 13.5% at 18 h. Kinetics, as per the Monod's                  |
| 727 | model and with ammonium being the substrate, also supported that result. At similar                  |
| 728 | temperatures and MLSS concentrations, the K value, or the half-saturation constant for the           |
| 729 | PO increased by 190.4% at 18 h and by 324.1% at 12 h. However, this research did not                 |
| 730 | analyze the complex relationship between the K value, the MLSS concentrations and the                |
| 731 | applied HRT. When air was used, the $k_H$ value, or the hydrolysis constant, decreased and           |
| 732 | the $k_d$ value, or the decay constant, increased, which additionally stated the advantage of        |
| 733 | the PO use, which was further strengthened at longer HRTs. In addition, by employing                 |
| 734 | oxidation of anaerobically treated excess sludge with PO, Zupančič and Roš (2008) showed             |
| 735 | that at an HRT of 5 days only 38% of ammonium was converted. This increased to $<70\%$ at            |
| 736 | HRTs of 6 days or 7 days, to 85.1% at 8 days and finally 85.6% at 10 days indicating that            |
| 737 | no further improvement was possible. As such, the nitrification bacteria needed longer               |
| 738 | contact times to process large loadings of ammonium.                                                 |
|     |                                                                                                      |

# 6. PURE OXYGEN AERATION AND MEMBRANE FOULING IN PURE OXYGEN MEMBRANE BIOREACTORS

As membrane fouling continues being an important research field in membrane bioreactors. 743 744 in this section, we mainly concentrate on the potential effect of PO aeration on its 745 mitigation during WWT. Despite the fact that the performance of MBRs operating at low/medium COD loading rates, e.g. <1 kg m<sup>-3</sup> d<sup>-1</sup>, has been widely studied, operation at 746 COD loading rates >2 kg m<sup>-3</sup> d<sup>-1</sup> is not common. This is because of the incapability of 747 maintaining a healthy aerobic environment, due to limited oxygen transfer efficiency, (Lee 748 749 and Kim, 3003). As such, PO aeration suits well MBRs where high MLSS concentrations have to be maintained and high organic loadings have to be treated, so OURs as high as 50 750  $mg_{O2}L^{-1}h^{-1} - 150 mg_{O2}L^{-1}h^{-1}$  are needed, (Larrea et al., 2014). 751 Although MBRs have many advantages over CAS, membrane fouling is still a problem that 752 increases running cost and potentially reduces treated water volumes and quality, (Zhuang 753 et al, 2016b). Membrane fouling in MBRs is the systematic accumulation of suspended 754 solids, colloids, and macromolecules on the membrane surface, or inside the pores, causing 755 a reduction in membrane permeability. It is a complex phenomenon that is affected by the 756 757 membrane itself, the wastewater under treatment, etc. with viscosity of wastewater, EPS/SMP production, size distribution of sludge flocs and the presence of colloids 758 contributing to this. An accurate indicator of the degree of membrane fouling is the 759 760 transmembrane pressure (TMP), (Zhuang et al., 20016b; Lee and Kim, 2003). With respect to the aeration type, Zhuang et al. (2016b) showed that the degree of 761 membrane fouling was 33% less (on day 90) when an MBR was fed with PO, instead of air. 762 Even though it had been expected that the higher air flowrates and the larger bubble size 763

| 764 | would have improved scouring and led to less fouling, it was finally proved that the PO fine                                                                    |
|-----|-----------------------------------------------------------------------------------------------------------------------------------------------------------------|
| 765 | bubbles controlled EPS production and the biomass particle size distribution better                                                                             |
| 766 | mitigating membrane fouling. Despite the fact that EPS amounts increased under both                                                                             |
| 767 | aeration types, their amount was lower when PO was used. High shear forces and high                                                                             |
| 768 | aeration intensities when air was used released more EPS deteriorating membrane fouling.                                                                        |
| 769 | Shear forces affect floc size, strength and compactness affecting sludge filterability. When                                                                    |
| 770 | PO was used, the flocs were constantly exposed to low shear forces, so they grew into loose                                                                     |
| 771 | weak ones. On the other hand, when air was used, the high shear forces broke flocs to                                                                           |
| 772 | fragments promoting membrane fouling, (Zhuang et al., 2016b).                                                                                                   |
| 773 | Lee and Kim (2003) also operated an MBR fed with PO at 2 $kg_{COD}$ m <sup>-3</sup> d <sup>-1</sup> and the critical                                            |
| 774 | TMP (50 kPa) was reached in 50 days. By increasing the OLR to 4 $kg_{\rm COD}m^{-3}d^{-1},$ it took 20                                                          |
| 775 | days, to 8 kg <sub>COD</sub> m <sup>-3</sup> d <sup>-1</sup> 10 days and finally to 10 kg <sub>COD</sub> m <sup>-3</sup> d <sup>-1</sup> less than 10 days. The |
| 776 | membrane fouling rates demonstrated a trend twice as high as the organic loadings,                                                                              |
| 777 | however, after applying 8 $kg_{COD}$ m <sup>-3</sup> d <sup>-1</sup> , the slope of membrane fouling rate decreased. As                                         |
| 778 | the organic loading increases, biomass also increases, so the F/M ratio is affected. The F/M                                                                    |
| 779 | ratio has additionally an effect on membrane fouling, but potential differences in MLSS                                                                         |
| 780 | concentrations and membrane fluxes at different F/M ratios have a complex effect, which is                                                                      |
| 781 | difficult to determine. For MLSS concentrations up to 10000 mg L <sup>-1</sup> , Kim et al. (2003)                                                              |
| 782 | found that the increase in TMP was smooth. However, it dramatically changed when the                                                                            |
| 783 | MLSS concentration exceeded 10000 mg L <sup>-1</sup> , as the fouling rate was almost four times                                                                |
| 784 | higher — it increased from 1.3 to 5.6. This became ten times higher, when the MLSS                                                                              |
| 785 | concentration exceeded 15000 mg L <sup>-1</sup> , as it increased from 1.3 to 12.5.                                                                             |
| 786 | MLSS concentrations directly affect viscosity. High viscosity suspensions require high                                                                          |
| 787 | cross flow velocities to create turbulence. If turbulence provided by aeration is not enough                                                                    |

788 to scour solids off the membrane, membrane becomes fouled faster. An optimal MLSS concentration in relation with F/M, viscosity and structure/size of flocs seems to exist. In 789 reality, membrane fouling is more affected by an increase in MLSS concentration than an 790 791 increase in the organic loading. However, it is difficult to evaluate membrane fouling by 792 disconnecting the two, as each time the organic loading increases, the biomass concentration increases as well. Therefore, it depends on whether membrane fouling is 793 794 assessed from an organic loading or a biomass point of view. However, membrane fouling does not start building up soon after an increase in the organic loading but it takes some 795 796 time due to biomass also needing time to increase and produce EPS at a sufficient amount. 797 However, it must be pointed that the opposite can also be found in the literature, namely MLSS concentrations have no effect or even have a positive one on TMP. This can indeed 798 happen, as the effect of MLSS concentrations on the filtration resistances is case-specific, 799 (Lee and Kim, 2003). 800

Finally, Rodriguez et al. (2012a) quantified the influence of PO or air on the recovery of 801 permeability of the membrane, namely the fraction of the difference of permeability, which 802 is the quotient of flux over TMP, after cleaning minus the permeability before cleaning 803 over permeability after cleaning. As such, a physical cleaning based on backflush of 804 805 permeate and a chemical one, which becomes necessary when irreversible fouling needs to 806 be removed, were tested. The mean recovery was found to be higher when PO was used regardless of the type of cleaning - 2% and 15% further improvement for the physical and 807 the chemical cleaning respectively, which is also in line with the fact that physical 808 cleanings are weaker than the chemical ones. 809

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#### 812 7. DISCUSSION AND FUTURE RESEARCH DIRECTIONS

813

PO aeration in aerobic WWT treatment, leads to higher OTEs at lower flowrates. It allows 814 815 faster treatment under higher MLSS concentrations and shorter HRTs. In fact, it can 816 promote treatment in cases where conventional aeration fails. EPS production was found to 817 be affected by the type of aeration. Under substrate utilization conditions, PO leads to the 818 production of more EPS, whereas under endogenous respiration, decomposition of 819 produced EPS is promoted. The latter additionally supports the findings that MBRs fed 820 with PO end up with less EPS than MBRs fed with air, as MBRs operate at longer SRTs to 821 reduce sludge production. However, the less EPS in MBRs fed with PO are also due to the lower PO flowrates, compared with equivalent air flowrates in MBRs fed with air, as the 822 lower PO flowrates reduce shear forces and flocs do not break. PO aeration under substrate 823 utilization conditions additionally improves aerobic sludge granulation, as PO increases 824 EPS production so that granules difficult to disintegrate are created. SMP production under 825 high F/M ratios is also higher when PO is used, as higher amounts of utilization associated 826 SMP are produced. On the other hand, the production of much larger MW biomass 827 associated SMP, which mainly comprise the effluent soluble organic matter, at low/medium 828 829 F/Ms, explains the higher refractory character of effluents, when PO is used. Enzymatic activity is accelerated during PO aeration. However, not all enzymes are benefited by the 830 use of PO, important hydrolytic enzymes remain unaffected by the aeration type. In 831 832 addition, the type of aeration does not significantly affect the bacterial diversity as well, however, it affects the relative abundance of the dominant bacteria. Finally, PO aeration 833 also satisfactorily controls foam formation in aeration tanks, due to the lower flowrates 834 needing to be applied. 835

836 Regarding removal of contaminants and treated water quality, PO aeration was proven quite efficient. However, after comparing performances of CAS and POAS systems with 837 respect to carbonaceous matter removal, it was found that the type of aeration was not so 838 839 critical as the HRT or the SRT. In addition, at low/medium F/M ratios, where both activated sludge systems can be equally applicable, no significant differences in terms of 840 organic carbon removal are reported. The advantage of the use of PO though is that 841 842 operation of POAS plants can be extended to higher F/M ratios where CAS plants are not usually designed to operate. Even though there is some research, which claims that, under 843 844 similar operating conditions, PO aeration may lead to improved removal of organic carbon, 845 this research is currently restricted to batch applications, e.g. SBRs, at small scale, where any improved removal efficiency can be attributed to the fact that PO acts faster than air. 846 However, PO use may be more promising with respect to the removal of refractory 847 pollutants and micropollutants. Under PO aeration, improved biological performance has 848 been monitored with respect to removal of phenolic micropollutants, antibiotics, endocrine 849 disrupting compounds, etc. Finally, organic matter removal in POAS plants remains 850 unaffected by the presence of toxic heavy metals in the wastewater. 851 Regarding nitrification, it mainly depends on parameters other than the type of aeration, 852 853 however, under conditions that undisputedly do not inhibit nitrification, higher DO 854 concentrations due to PO, improved the removal of ammonium from wastewaters. 855 Nonetheless, POAS systems are designed to operate at short SRTs and remove only organic 856 carbon, as the short SRTs hinder nitrification. This can be overcome by continually seeding 857 the aeration tanks with aged sludge. In addition, under conditions where PO alternation with air, or vice versa, is possible, simultaneous removal of carbon and ammonia in one 858 basin may also possible. If an extra treatment step for successful nitrification cannot be 859

avoided, MBBRs fed with PO have been suggested for tertiary nitrification of secondary
effluent. In any other event, MBRs, which, by definition, operate at longer SRTs, have to
be considered, with MBRs fed with PO achieving better nitrification rates than those fed
with air.

Regarding MBRs fed with PO, they may be proven to be quite useful in cases where high 864 organic loadings are combined with high MLSS concentrations. MBRs fed with PO remove 865 866 satisfactorily both carbonaceous and nitrogenous matter as well as micropollutants. In addition, the use of PO contributes to membrane fouling mitigation. In MBRs fed with PO, 867 868 the lower flowrates that have to be applied control better any increase in TMP values, as the 869 lower shear forces do not break flocs, so less EPS are produced. However, as organic loadings increase, MBRs fed with PO can become more vulnerable to membrane fouling. 870 Finally, better recovery of permeability is achieved in MBRs fed with PO than in MBRs fed 871 with air after application of a cleaning, with chemical cleanings having always a better 872 873 performance than physical cleanings regardless the type of aeration. 874 Even though direct comparison of the two aeration types is not an easy task, there are cases where the use of PO does have a significant advantage over air, e.g. when high strength 875 876 wastewaters have to be treated. In these cases, PO use is unavoidably recommended. 877 However, the field still needs investigation, as the available literature is currently dominated by the use of air. Based on what most specifically has been found in this work, 878 future research should concentrate on whether or not POAS can remove contaminants that 879 usually are not removed by CAS and on analyzing further the effect of PO on the enzyme 880 activity as all enzymes are not equally affected by the type of aeration. Finally, the potential 881 for operation of MBRs fed with PO at higher organic loadings and higher MLSS 882 concentrations has to be further determined. 883

## 884 8. CONCLUSION

| 005 |                                                                                              |  |  |  |
|-----|----------------------------------------------------------------------------------------------|--|--|--|
| 886 | PO achieves faster treatment rates at higher biomass concentrations and shorter HRTs. It     |  |  |  |
| 887 | better controls EPS/SMP production, accelerates enzyme activity, produces less sludge and    |  |  |  |
| 888 | minimizes foam. In MBRs fed with PO, it also better controls membrane fouling and            |  |  |  |
| 889 | improves recovery of permeability after cleanings. PO is recommended when high strength      |  |  |  |
| 890 | wastewaters are treated. However, PO use has also been connected with some problems,         |  |  |  |
| 891 | such as the pH drop in the mixed liquor in the closed headspace POAS systems. Finally, PO    |  |  |  |
| 892 | may also produce final effluents of a higher refractory character. It is recommended that PO |  |  |  |
| 893 | be considered where air fails.                                                               |  |  |  |
| 894 |                                                                                              |  |  |  |
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## **TABLE 1:** Research studies based on PO aeration

| Case Study                   | Type of Wastewater                  |       |
|------------------------------|-------------------------------------|-------|
| Bébin and Renaudin, 1976     | Dairy Industry Waste Effluent       | Pilot |
| Calderón et al., 2012        | Primary Effluent                    | Pilot |
| Canziani et al., 2006        | Leachate                            | Pilot |
| Collivignarelli et al., 2015 | Industrial Wastewater               | Full  |
| Dorival-García et al., 2014  | Domestic Wastewater                 | Pilot |
| Esparza-Soto et al., 2006    | Primary Effluent                    | Bench |
| Hu et al., 2019              | Hypersaline Wastewater              | Bench |
| Lee and Kim. 2003            | Synthetic Wastewater                | Bench |
| Lee et al. 2003              | Municipal + Industrial Wastewater   | Pilot |
| Mauret et al. 2001           | Food Processing Wastewater          | Full  |
| Moerman et al. 1995          | Pretreated Carbonization Wastewater | Full  |
| Neethling et al., 1998       | Municipal Wastewater                | Full  |
| Pan et al., 2017             | High-Loading Wastewater             | Bench |
| Peterson et al., 1978        | Mill Effluent                       | Full  |
| Rempel et al., 1992          | Bleached Kraft Pulp Mill Effluent   | Pilot |
| Rodríguez et al. 2010        | Primary Effluent                    | Bench |
| Sears et al., 2003           | Primary Effluent                    | Bench |
| Verstraete, 1980             | Petrochemical Water                 | Full  |
| Zhang et al., 2019           | Synthetic Wastewater                | Bench |
| Zhuang et al. 2016a,b        | Coal Gasification Wastewater        | Bench |
| Zupančič and Roš, 2008       | Excess Sludge                       | Bench |
| 1113                         |                                     |       |
|                              |                                     |       |

## **TABLE 2:** Industrial WWT POAS applications

| Reference                                       | Application                                                                                                                                                                    | Flowrate or Capacity                                 |
|-------------------------------------------------|--------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|------------------------------------------------------|
| Batt et al., 2007                               | Lackawanna WWT Plant, Erie County, New York, USA                                                                                                                               | $17\ 034\ m^3\ d^{-1}$                               |
| Brand et al., 2019                              | WWT Plant near Stanford, Northern California, USA                                                                                                                              | 240 000 m <sup>3</sup> d <sup>-1</sup>               |
| Collivignarelli et al., 2015                    | WWT Plant, Northern Italy, Italy                                                                                                                                               | 280 m <sup>3</sup> d <sup>-1</sup> (Average)         |
| Confer et al., 1995; Esparza-Soto et al., 2006a | Ina Road Water Pollution Control Facility, Tucson, Arizona, USA                                                                                                                | N/A                                                  |
| Gostick et al., 1992                            | Christian Salvesen (Food Services) Ltd Bourne, Lincolnshire, UK                                                                                                                | 3303 m <sup>3</sup> (Basin Volume)                   |
| Jolis et al., 2006                              | Oceanside Water Pollution Control Plant, County and City of San                                                                                                                | 0.7 m <sup>3</sup> s <sup>-1</sup> (Average DWF)     |
| Jons et al., 2000                               | Francisco, California, USA                                                                                                                                                     | 2.8 m <sup>3</sup> s <sup>-1</sup> (Average WWF)     |
| Karibayashi, 1992                               | Todoroki Sewerage Works, Kawasaki, Japan                                                                                                                                       | 395 500 m <sup>3</sup> d <sup>-1</sup>               |
| Kundral et al., 2015                            | South District WWT Plant, Miami-Dade County, Florida, USA                                                                                                                      | $4.9 \text{ m}^3 \text{ s}^{-1}$                     |
| Loiacono et al., 1992                           | Southeast Water Pollution Control Plant (SEP), San Francisco, USA                                                                                                              | 3.5 m <sup>3</sup> s <sup>-1</sup>                   |
| Marshal and Sousley, 1997                       |                                                                                                                                                                                | 1.5 m <sup>3</sup> s <sup>-1</sup> (Design Load)     |
| Marshar and Sousley, 1997                       | Simpson Tacoma Kraft Effluent Treatment Facility                                                                                                                               | 1.7 m <sup>3</sup> s <sup>-1</sup> (Max. Load)       |
| Mauret et al., 2001                             | Food-Processing Industry WWT Plant (Slaughterhouse), France <sup>a</sup>                                                                                                       | 3500 PE                                              |
| Mines, 1992                                     | Main Street Wastewater Treatment Plant, Pensacola, Florida, USA                                                                                                                | 0.9 m <sup>3</sup> s <sup>-1</sup>                   |
| Moerman et al., 2008                            | N/A                                                                                                                                                                            | $80 \text{ m}^3 \text{ h}^{-1}$ (after 50% Dilution) |
| Neethling et al., 1998                          | Rock Creek WWT Plant, Portland, Oregon, USA                                                                                                                                    | 3500 m <sup>3</sup> (Basin Volume)                   |
| Peterson et al., 1978                           | Longview Fiber's Mill Treatment System, Longview, Washington, USA                                                                                                              | 2.6 m <sup>3</sup> d <sup>-1</sup>                   |
| Randal and Cokgor, 2001                         | HENRICO County, Virginia, Water Reclamation Facility, USA <sup>b</sup>                                                                                                         | 170 325 m <sup>3</sup> d <sup>-1</sup>               |
| Sears et al., 1995                              | North End Wastewater Pollution Control Centre, Winnipeg, Canada                                                                                                                | N/A                                                  |
| Verstraete, 1980                                | BP Chemicals Belgium Works WWT, Belgium 3000                                                                                                                                   |                                                      |
|                                                 | Veather Flow, N/A: Non-Applicable, PE: Population Equivalent, UK: United F<br>ca, WWF: Wet Weather Flow. <sup>a</sup> Alternating Anoxic-Aerobic Process, <sup>b</sup> Biologi |                                                      |

| Case Study                   | Treatment Method              | Pollutant                      | Removal Efficiency: Air (%) | Removal Efficiency: PO (%) |
|------------------------------|-------------------------------|--------------------------------|-----------------------------|----------------------------|
| D ( 1 2015                   | Activated Sludge + Fenton     | SCOD                           |                             | 66                         |
| Bae et al., 2015             | Oxidation Unit                | Color                          |                             | 73                         |
| Bonomo et al., 2000          | MBBR                          | NH4 <sup>+</sup> -N            |                             | >80                        |
| Collivignarelli et al., 2015 | MBR + NF Unit                 | COD                            |                             | 90                         |
|                              |                               | COD                            |                             | 90                         |
| Dorival-Garcia et al., 2014  | MBR                           | TSS                            |                             | 97                         |
|                              |                               | TN                             |                             | 90                         |
| M 1 2010                     |                               | TOG                            | 28.8% (Salinity: 0.5%)      | 78.1 (Salinity: 0.5%)      |
| Hu et al., 2019              | Activated Sludge <sup>a</sup> | TOC                            | 13.3% (Salinity: 5%)        | 17.2 (Salinity: 5%)        |
| L 117: 2002                  |                               |                                |                             | up to 99.7 (Synthetic)     |
| Lee and Kim, 2003            | Anoxic Reactor + MBR          | COD                            |                             | up to 97.5 (Leachate)      |
|                              |                               | COD (Granules)                 | ~80                         | 91                         |
| D ( 1 2017                   |                               | NH4 <sup>+</sup> -N (Granules) | >80                         | 80                         |
| Pan et al., 2017             | Activated Sludge              | COD (Flocs)                    | >80                         | <80                        |
|                              |                               | NH4 <sup>+</sup> -N (Flocs)    | ~75                         | ~80                        |
| D 1 4 1 1000                 |                               | BOD <sub>5</sub>               | 79 - 96                     | 71 - 94                    |
| Rempel et al., 1992          | Activated Sludge              | COD                            | 27 - 50                     | 29 - 56                    |
| D 1: (1.2010                 |                               | COD                            |                             | >90                        |
| Rodriguez et al., 2010       | MBR                           | BOD <sub>5</sub>               |                             | >90                        |
| D 1                          | MDD                           | NH4 <sup>+</sup> -N            | 65.5±11.9 (HRT:18 h)        | 79.1±9.7 (HRT:18 h)        |
| Rodriguez et al., 2012c      | MBR                           |                                | 59±20.1 (HRT: 12 h)         | 67±8.7 (HRT: 12 h)         |
| Sears et al., 2003           | Activated Sludge              | NH4 <sup>+</sup> -N            |                             | >90                        |
| Zhang et al., 2019           | Activated Sludge <sup>a</sup> | TOC                            | 35.9                        | 87.3                       |
| 71 1 2016                    |                               | COD                            | 52                          | 90                         |
| Zhuang et al., 2016a         | MBR                           | TPh                            | 50                          | 95                         |
| Zupančič and M. Roš, 2008    | Apropia Transmost of Shud     | COD                            | 55 (18 d), 55 (23 d)        | 55 (18 d), 60 (23 d)       |
| Lupancie and M. Kos, 2008    | Aerobic Treatment of Sludge   | VSS                            | 60 (21 d)                   | 60 (39 d)                  |
|                              |                               |                                |                             |                            |

## **TABLE 3:** Pollutant removal efficiencies as indicated in a number of selected studies

**1130** Abbreviations: BOD<sub>5</sub>: Five-day Biological Oxygen Demand, COD: Chemical Oxygen Demand, HRT: Hydraulic

**1131** Residence Time, MBBR: Moving Bed Biofilm Reactor, MBR: Membrane Bioreactor, NF: Nanofiltration, NH<sub>4</sub><sup>+</sup>-N:

1132 Ammonium Nitrogen, SCOD: Soluble COD, TOC: Total Organic Carbon, TN: Total Nitrogen, TPh: Total Phenols, VSS:

1133 Volatile Suspended Solids. <sup>a</sup>Batch Reaction